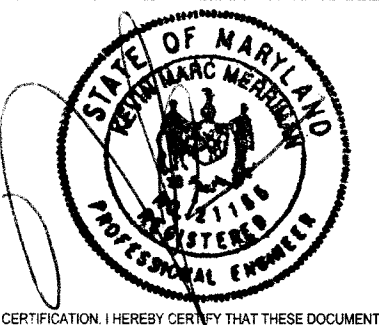




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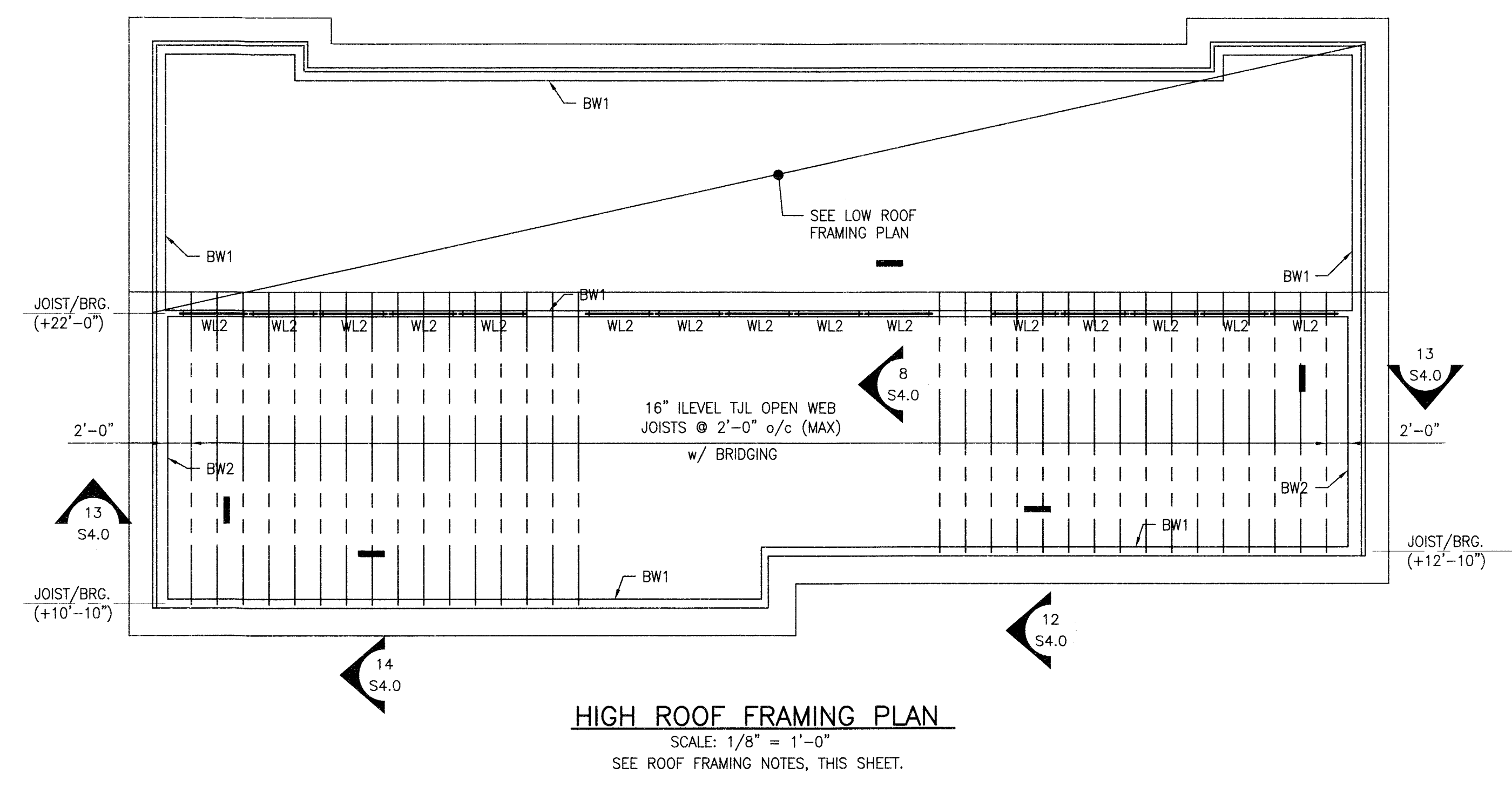
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ROOF FRAMING PLANS

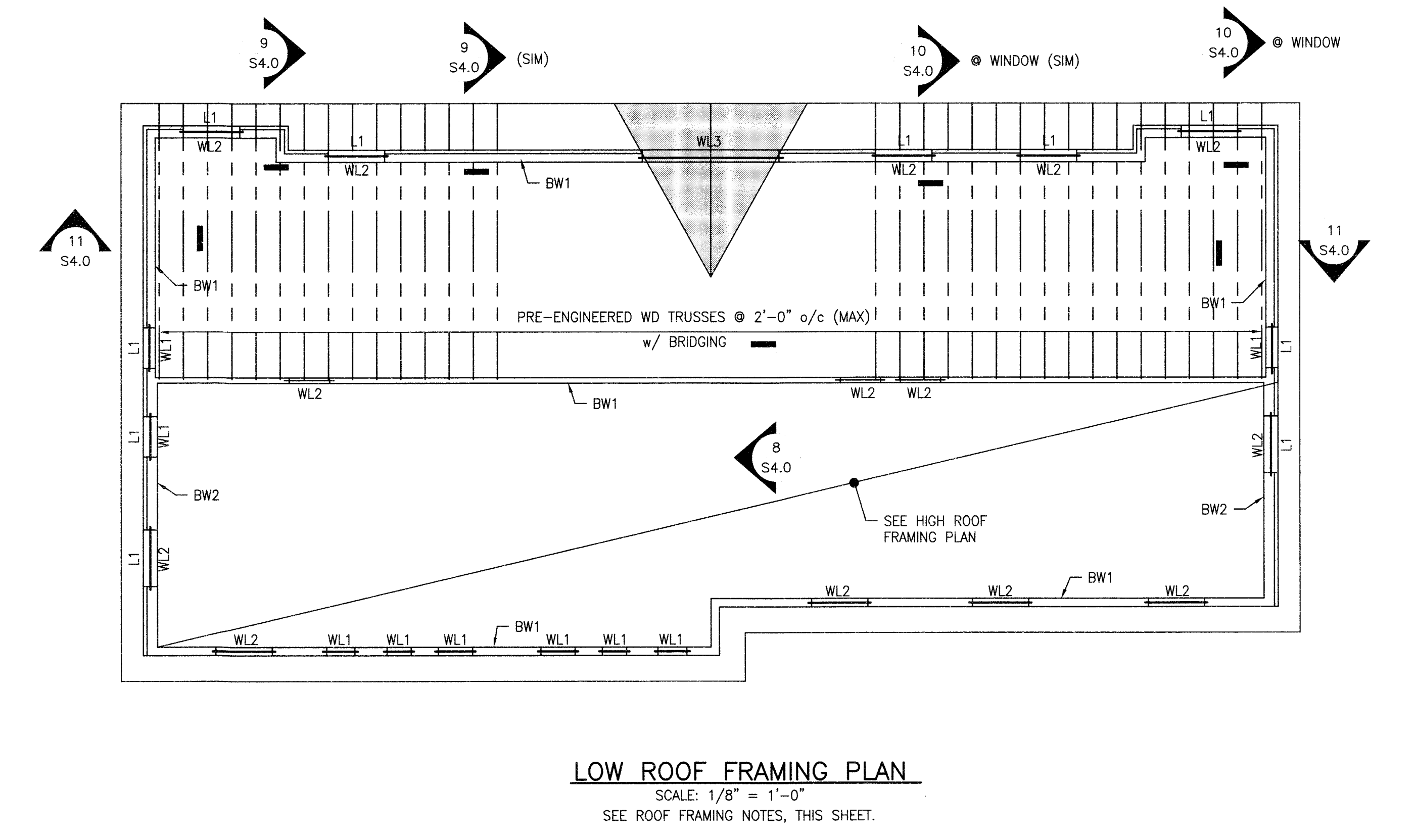
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- ROOF FRAMING NOTES:
1. ROOF SHEATHING OVER WOOD TRUSSES SHALL BE 5/8" APA RATED OSB SHEATHING, GROUP I, EXTERIOR, PANEL ID 48/24.
 2. ROOF SHEATHING OVER TJI OPEN WEB JOISTS SHALL BE 5/8" APA RATED OSB SHEATHING, GROUP I, EXTERIOR, PANEL ID 48/24.
 3. WOOD TRUSS BOTTOM CHORD BEARING ELEVATION AT EXTERIOR WALLS = +9'-4" UNLESS NOTED OTHERWISE ON PLAN THUS (+X'-XX").
 4. SHEATH EXTERIOR WALLS WITH 7/16" APA RATED OSB SHEATHING, EXTERIOR, STRUCTURAL I GRADE. ATTACH TO STUDS w/ 8d NAILS @ 6"/c AT PANEL EDGES AND 12"/c AT INTERMEDIATE FRAMING MEMBERS. SEE GENERAL NOTES FOR PRE-ENGINEERED WOOD ROOF TRUSS AND WOOD TRUSS GIRDER DESIGN LOADS, BRACING, AND BRIDGING REQUIREMENTS. DESIGN FOR L/240 TOTAL LOAD DEFLECTION.
 5. WLX DENOTES WOOD LINTEL. SEE SCHEDULE ON S2.0. PROVIDE LINTELS IN INTERIOR WALLS PER LINTEL SCHEDULE. SEE ARCH DWGS FOR WALL OPENINGS NOT SHOWN.
 6. ALL WOOD LINTELS SHALL HAVE POSTS PER THE WOOD LINTEL SCHEDULE. POSTS FROM FLOORS ABOVE SHALL BE CONTINUOUS TO THE SLAB-ON-GRADE OR TRANSFER BEAM.
 7. THE CONTRACTOR SHALL COORDINATE THE LOCATION OF ALL OPENINGS, PENETRATIONS, MECHANICAL UNITS, DUCTWORK, CONDUITS, LINTELS, ETC. WITH THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS.
 8. □ DENOTES TRUSS OVER-BUILD.
 9. PROVIDE SIMPSON OR EQUIVALENT HURRICANE TIES AT ALL WOOD ROOF TRUSS BEARING LOCATIONS.

MARK	WOOD BEARING WALL SCHEDULE	
	FLOOR	REMARKS
BW1	2x6 @ 16" OC	
BW2	2x8 @ 16" OC	

- BEARING WALL SCHEDULE NOTES:
1. PROVIDE CONTINUOUS, DOUBLE TOP PLATE ON ALL BEARING WALLS
 2. PROVIDE CONTINUOUS WOOD BLOCKING AT 4'-0" OC MAX. ALIGN CONTINUOUS BLOCKING WITH SHEATHING PANEL EDGES.
 3. WOOD FOR BEARING WALLS SHALL BE SPF #1/#2 OR BETTER. SEE GENERAL NOTES FOR ADDITIONAL INFORMATION





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DESIGN CRITERIA

- DESIGN GRAVITY LIVE LOADS:

AREA	LIVE LOAD
SLAB-ON-GRADE/FIRST FLOOR ROOF	100 PSF 30 PSF
- SNOW LOADING IBC 2006

GROUND SNOW LOAD	25 PSF
FLAT ROOF SNOW LOAD	20 PSF + SNOW DRIFT
SNOW EXPOSURE FACTOR	0.9
SNOW IMPORTANCE FACTOR	1.0
THERMAL FACTOR	1.0
- WIND LOADING IBC 2006

BASIC WIND SPEED (3 SECOND GUST)	90 MPH
WIND LOAD IMPORTANCE FACTOR	1.0
WIND EXPOSURE CATEGORY	B
INTERNAL PRESSURE COEFFICIENT	0.18
COMPONENT AND CLADDING PRESSURE (TRIB AREA 10 SQ. FT.)	
ZONE 1	+14.5/-26.9 PSF
ZONE 2	+14.5/-33 PSF
ZONE 3	+10.1/-34 PSF
ZONE 4	+14.5/-15.8 PSF
ZONE 5	+14.5/-19.5 PSF
- SEISMIC LOADING IBC 2006

SEISMIC USE GROUP	II
Ss	0.152 G
S1	0.050 G
SDS	0.163 G
SD1	0.080 G
SITE CLASS	D
SEISMIC IMPORTANCE FACTOR	1.00
SEISMIC DESIGN CATEGORY	B
- ANY MECHANICAL EQUIPMENT NOT SHOWN ON THE STRUCTURAL DRAWINGS AND HAVING A WEIGHT IN EXCESS OF 500 POUNDS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER PRIOR TO INSTALLATION.

SUBMITTALS

- BEFORE SUBMISSION OF SHOP DRAWINGS, THE CONTRACTOR SHALL HAVE DETERMINED AND VERIFIED ALL QUANTITIES, DIMENSIONS, SPECIFIED PERFORMANCE CRITERIA, INSTALLATION REQUIREMENTS, MATERIALS, CATALOG NUMBERS AND SIMILAR DATA AND SHALL HAVE COORDINATED EACH SHOP DRAWING WITH OTHER SHOP DRAWINGS AND THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.
- PRIOR TO SUBMISSIONS, THE CONTRACTOR SHALL STAMP OR PROVIDE A SIMILAR WRITTEN INDICATION THAT THE CONTRACTOR HAS REVIEWED THE SUBMISSION AND IS SATISFIED THE CONTENTS ARE IN COMPLIANCE WITH THE CONTRACT DOCUMENTS.
- REPRINTS OF THE CONTRACT DOCUMENTS WILL NOT BE ACCEPTED.
- NO DIMENSIONAL INFORMATION MAY BE OBTAINED BY DIRECT SCALING OF THE DRAWINGS.
- ADEQUATE SETS SHALL BE SUBMITTED SO THAT THE ARCHITECT /ENGINEER CAN MAINTAIN ONE RECORD SET AT ALL TIMES.

FOUNDATIONS

- ALL SPREAD FOOTINGS SHALL BEAR ON UNDISTURBED SOIL OR CONTROLLED STRUCTURAL FILL, HAVING A MINIMUM SAFE BEARING CAPACITY OF 3,000 PSF. ALL SPREAD FOOTINGS SHALL PROJECT AT LEAST 1'-0" INTO SOIL HAVING SUCH MINIMUM BEARING VALUE.
- RETAIN THE SERVICES OF A REGISTERED GEOTECHNICAL ENGINEER, APPROVED BY THE ARCHITECT AND PAID FOR BY THE OWNER, TO VERIFY SOIL BEARING CAPACITY AT EACH FOOTING PRIOR TO INSTALLATION. NOTIFY ARCHITECT/ENGINEER OF ANY VARIATION FROM ANTICIPATED BEARING CAPACITY FOR APPROPRIATE REDESIGN OR LOWERING OF FOOTINGS.
- EXCAVATION, PREPARATION OF SUBGRADE, AND FOOTING CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT BY ECS, LLC AND DATED SEPTEMBER 10, 2009
- COMPACT FILL AND BACKFILL TO 95% OF A.A.S.H.T.O. T-180. ALL FILL AND BACKFILL OPERATIONS SHALL BE PERFORMED UNDER THE DIRECT SUPERVISION OF THE GEOTECHNICAL ENGINEER.
- ALL ORGANIC MATERIALS AND CONSTRUCTION DEBRIS SHALL BE REMOVED IN REGIONS OF ALL FOUNDATIONS.
- THE BOTTOMS OF ALL EXTERIOR FOOTINGS SHALL BE 3'-0" MINIMUM BELOW FINISHED GRADE.
- EDGES OF FOOTINGS SHALL NOT BE PLACED AT A GREATER THAN 1 (VERTICAL) TO 2 (HORIZONTAL) SLOPE WITH RESPECT TO ANY ADJACENT FOOTING OR EXCAVATION.
- THE CONTRACTOR SHALL SAFEGUARD AND PROTECT ALL EXCAVATIONS, AND ALL EXCAVATIONS SHALL BE KEPT FREE OF WATER.
- NO HORIZONTAL JOINTS SHALL BE PLACED IN WALLS EXCEPT AS SHOWN ON THE DRAWINGS WITHOUT APPROVAL OF THE ARCHITECT/ENGINEER.
- THE CONTRACTOR SHALL REFER TO THE ARCHITECTURAL, PLUMBING, MECHANICAL AND ELECTRICAL DRAWINGS FOR ALL LOCATIONS OF TRENCHES, PITS, CONDUITS, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- BACKFILLING AGAINST WALLS SHALL NOT BE DONE UNTIL CONCRETE AND/OR MASONRY GROUT HAS BEEN CURED TO ATTAIN SUFFICIENT STRENGTH (7 DAYS MINIMUM) AND WALLS ARE PROPERLY SHORED AND/OR BRACED. BACKFILLING AGAINST BASEMENT WALLS SHALL NOT BE DONE UNTIL THE FLOOR SLABS AT TOP AND BOTTOM OF WALLS HAVE BEEN PLACED AND HAVE CURED. BACKFILL FOUNDATION WALLS WITH EARTH ON BOTH SIDES OF THE WALL BY ALTERNATELY PLACING BACKFILL ON EACH SIDE SO THAT HEIGHT OF BACKFILL DOES NOT DIFFER BY MORE THAN 1'-6" FROM OTHER SIDE.
- ALL ADJACENT COLUMN FOOTINGS THAT ABUT SHALL BE SEPARATED BY A PAPER JOINT.

FOUNDATION CONCRETE

- ALL CONCRETE SHALL CONFORM TO THE PROVISIONS OF ACI BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318-LATEST EDITION) AND ACI SPECIFICATIONS FOR STRUCTURAL CONCRETE IN BUILDINGS (ACI 301-LATEST EDITION).
- ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE HAVING A DESIGN COMPRESSIVE STRENGTH AT 28 DAYS AS FOLLOWS:

A. FOOTINGS AND PIERS	3,000 PSI
B. SLAB ON GRADE	3,000 PSI
C. RETAINING WALLS	4,000 PSI
- NO CONCRETE SHALL BE PLACED UNTIL CONCRETE DESIGN MIXES HAVE BEEN SUBMITTED FOR EACH CLASS OF CONCRETE NOTED ABOVE AND HAVE BEEN REVIEWED BY THE ARCHITECT/ENGINEER.
- USE A WATER REDUCING ADMIXTURE IN ALL CONCRETE.
- PROVIDE MIX DESIGNS WITH A WATER-CEMENT RATIO FROM 0.40 TO 0.50.
- SLUMP SHALL BE AS REQUIRED BY ACI 302 - LATEST EDITION
- AIR ENTRAIN ALL CONCRETE EXPOSED TO WEATHER OR EARTH TO 5%±1%.
- NO CALCIUM CHLORIDE IN ANY FORM WILL BE PERMITTED IN CONCRETE.
- ALL STRUCTURAL MEMBERS SHALL BE POURED FOR THEIR FULL DEPTHS IN ONE OPERATION.
- EXCAVATIONS SHALL BE KEPT FREE OF WATER. NO CONCRETE SHALL BE PLACED IN WATER.
- ALL SLABS ON GRADE SHALL HAVE THICKENINGS, DEPRESSIONS, OPENINGS, ETC. AS SHOWN OR AS REQUIRED BY VARIOUS TRADES.
- REFER TO ARCHITECTURAL DRAWINGS AND/OR SPECIFICATION SECTIONS FOR CONCRETE FINISHES.
- RETAIN THE SERVICES OF AN INDEPENDENT TESTING AGENCY APPROVED BY THE ARCHITECT TO PERFORM TESTS OF CONCRETE. TAKE A MINIMUM OF 4 CYLINDERS FOR EACH CLASS OF CONCRETE POURED IN ANY ONE DAY. PERFORM 1 SLUMP TEST PER TRUCK LOAD OF CONCRETE.

CONCRETE MASONRY

- CONCRETE MASONRY CONSTRUCTION SHALL CONFORM TO ALL REQUIREMENTS OF BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530-LATEST EDITION) AND ACI SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530.1-LATEST EDITION).
- CONCRETE MASONRY UNITS (CMU) SHALL BE GRADE N, 2-CELL BLOCK WITH A MINIMUM COMPRESSIVE STRENGTH OF 1,900 PSI ON NET CROSS SECTION UNLESS OTHERWISE NOTED. CMU SHALL BE TYPE I FOR EXTERIOR AND FOUNDATION WALLS. TYPE I OR TYPE II MAY BE USED FOR INTERIOR PARTITION WALLS.
- CONCRETE MASONRY UNITS SHALL CONFORM TO THE REQUIREMENTS OF THE MOST RECENT EDITIONS OF THE FOLLOWING STANDARDS:

A. HOLLOW LOAD-BEARING UNITS	ASTM C90
B. SOLID LOAD-BEARING UNITS	ASTM C145
C. HOLLOW NON-LOAD-BEARING UNITS	ASTM C129
D. CONCRETE BUILDING BRICK	ASTM C55
- ALL CONCRETE MASONRY UNITS SHALL BE NORMAL WEIGHT.
- MORTAR FOR REINFORCED AND NONREINFORCED MASONRY SHALL CONFORM TO THE REQUIREMENTS OF ASTM C270 (LATEST EDITION), TYPE S.
- GROUT FOR REINFORCED OR NONREINFORCED MASONRY SHALL CONFORM TO THE REQUIREMENTS OF ASTM C476 (LATEST EDITION)
- WHERE DRAWINGS INDICATE CELLS OF CONCRETE MASONRY UNITS (CMU) ARE TO BE FILLED SOLID, CELLS OF CMU SHALL BE FILLED WITH 3000 PSI PEA GRAVEL CONCRETE IN SIX COURSE MAXIMUM LIFTS.
- WHERE DRAWINGS SPECIFY GROUT TO BE PROVIDED, 3000 PSI PEA GRAVEL CONCRETE CAN BE SUBSTITUTED IF APPLICABLE TO USE (3000 PSI PEA GRAVEL CONCRETE SHALL NOT BE USED UNDER COLUMN BASEPLATES OR BEARING PLATES).
- ALL REINFORCEMENT SHOWN IN WALLS SHALL BE CENTERED IN MASONRY UNITS UNLESS NOTED OTHERWISE.
- ALL BLOCK SHALL HAVE GALVANIZED, TRUSS OR LADDER TYPE, HORIZONTAL JOINT REINFORCEMENT AT 16" O/C MAXIMUM WITH PREFABRICATED CORNER AND "T" PIECES UNLESS NOTED OTHERWISE. LAP ALL SPICES 6" MINIMUM. PROVIDE AN ADDITIONAL ROW ABOVE AND BELOW ALL OPENINGS AND EXTEND TWO FEET BEYOND JAMBS. STOP HORIZONTAL REINFORCING EACH SIDE OF CONTROL JOINTS.
- WHERE MASONRY WALLS OF HOLLOW UNITS OR MASONRY BONDED WALLS CHANGE IN THICKNESS, THE WALL SHALL BE FILLED SOLID WITH CONCRETE OR OTHERWISE CONSTRUCTED SOLID FOR AT LEAST ONE COURSE (8" MIN) IMMEDIATELY BELOW SUCH LEVEL WHERE THE THICKNESS CHANGES.
- EXCEPT AS OTHERWISE SHOWN, CELLS IN MASONRY UNDER BEARING AREAS FOR BEAMS, LINTELS, AND SLABS SHALL BE FILLED SOLID WITH CONCRETE FOR AT LEAST TWO COURSES (16" MIN) IMMEDIATELY BELOW SUCH BEARING.
- ALL MASONRY WALLS SHALL HAVE TEMPORARY BRACING INSTALLED UNTIL ALL STEEL AND DECK IS COMPLETE. CONTRACTOR IS SOLELY RESPONSIBLE FOR TEMPORARY BRACING.

REINFORCEMENT

- ALL DEVELOPMENT AND SPLICES OF REINFORCEMENT SHALL CONFORM TO THE PROVISIONS OF ACI BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318-LATEST EDITION).
- REINFORCING STEEL SHALL BE DEFORMED BARS OF INTERMEDIATE GRADE NEW BILLET STEEL CONFORMING TO CURRENT REQUIREMENTS OF ASTM A615 GRADE 60 EXCEPT TIES MAY BE GRADE 40. ALL HOOKS SHALL BE STANDARD HOOKS, UNLESS OTHERWISE NOTED.
- WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM A185 (LATEST EDITION).
- ALL WELDED WIRE FABRIC SHALL BE SPLICED SO THAT THE OVERLAP OF THE OUTERMOST CROSS WIRES OF EACH ADJOINING SHEET IS NOT LESS THAN THE SPACING OF THE CROSS WIRES PLUS TWO INCHES, UNLESS NOTED OTHERWISE.
- REINFORCING BAR SUPPORTS AND SPACERS SHALL CONFORM TO ACI 315-(LATEST EDITION) DETAILING MANUAL.
- SHOP DRAWINGS SHOWING ALL NECESSARY SECTIONS AND DETAILS FOR THE PROPER POSITIONING OF ALL REINFORCING STEEL SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW BEFORE FABRICATION OR PLACEMENT OF THE STEEL.

PRE-ENGINEERED WOOD ROOF TRUSSES

- DESIGN AND INSTALL TRUSSES AND CONNECTORS IN ACCORDANCE WITH APPLICABLE BUILDING CODE REQUIREMENTS AS WELL AS ANSI/TPI 1-2002 AS PUBLISHED BY THE TRUSS PLATE INSTITUTE, INC.
- INSTALL TEMPORARY AND PERMANENT BRACING OF WOOD TRUSSES IN ACCORDANCE WITH MANUFACTURERS DESIGN, BCSI 1, "BUILDING COMPONENT SAFETY INFORMATION", AND DSB-89 PUBLISHED BY THE TRUSS PLATE INSTITUTE (TPI), INC AND THE WOOD TRUSS COUNCIL OF AMERICA (WTCA).
- ALL TRUSS MEMBERS SHALL BE DESIGNED AND SIZED (INCLUDING GRADE AND SPECIES) AS REQUIRED TO SUPPORT THE LOADS INDICATED ON THE DRAWINGS (INCLUDING LIVE, DEAD, WIND, UPLIFT, AND MECHANICAL LOADS SUCH AS SPRINKLERS AND ROOF TOP UNITS). (19% MAX MOISTURE CONTENT IN USE).
- TRUSS TO TRUSS AND TRUSS TO HEADER CONNECTIONS AND HARDWARE SHALL BE DESIGNED AND PROVIDED BY THE TRUSS MANUFACTURER.
- SUBMIT TO ARCHITECT/ENGINEER, PRIOR TO FABRICATION, COMPLETE SHOP DRAWINGS FOR ALL TRUSSES. SHOP DRAWINGS SHALL INCLUDE STRESS DIAGRAMS, MEMBER GRADES AND SIZES, SIZE AND LOCATION OF ALL CONNECTOR PLATES, SIZE AND LOCATION OF ALL TEMPORARY BRACING, PERMANENT TRUSS BRIDGING AND MEMBER BRACING, DATA RELATIVE TO PREFABRICATED HANGERS FOR TRUSS TO TRUSS AND TRUSS TO HEADER CONNECTIONS, DESIGN COMPUTATIONS AND ERECTION PLANS. REPRINTS OF CONTRACT DRAWINGS ARE NOT ACCEPTABLE. DESIGN COMPUTATIONS AND SHOP DRAWINGS SHALL BE SIGNED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF THE PROJECT. ONLY SHOP DRAWINGS BEARING THE STAMP OF THE ARCHITECT SHALL BE USED FOR FABRICATION AND ERECTION.
- TEMPORARY MEMBER BRACING, PERMANENT TRUSS BRIDGING, WEB COMPRESSION MEMBER BRACING ETC., SHALL BE AS REQUIRED BY THE TRUSS MANUFACTURER'S DESIGN, AND SHALL BE INSTALLED BY CONTRACTOR IN ACCORDANCE WITH MANUFACTURER'S REQUIREMENTS.
- TEMPORARY BRACING SHALL REMAIN IN PLACE UNTIL THE TRUSSES ARE FULLY INSTALLED, PERMANENTLY BRACED AND SHEATHED AND ALL TRUSSES HAVE BEEN SECURELY ATTACHED TO THE SUPPORTS PER THE CONTRACT DOCUMENTS AND THE TRUSS MANUFACTURER'S RECOMMENDATIONS.
- DO NOT PROCEED WITH ROOF CONSTRUCTION UNTIL ALL TEMPORARY BRACING IS SECURELY AND PROPERLY IN PLACE AND HAS BEEN REVIEWED BY THE CONSTRUCTION INSPECTOR.
- PLACE PLYWOOD ROOF SHEATHING IN STACK HEIGHTS AND LOCATIONS ONLY AS DESCRIBED IN THE TRUSS MANUFACTURERS INSTALLATION LITERATURE AND PER THE TPI REFERENCES NOTED.
- PROVIDE ALL TRUSS FASTENERS AND HURRICANE CLIPS PER THE CONTRACT DOCUMENTS AND THE TRUSS MANUFACTURER'S RECOMMENDATIONS.
- CONTRACTOR SHALL EMPLOY AN INDEPENDENT INSPECTION AGENCY TO VISUALLY INSPECT TRUSSES BEFORE AND AFTER ERECTION. INSPECTION AGENCY SHALL CERTIFY THAT THE TRUSSES, CONNECTIONS AND BRACING HAVE BEEN INSTALLED IN COMPLIANCE WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.

WOOD FRAMING

- WOOD FRAMING AND FASTENERS - COMPLY WITH THE RECOMMENDATIONS OF THE AMERICAN FOREST AND PAPER ASSOCIATION.
- PLYWOOD - AMERICAN PLYWOOD ASSOC. (APA) GRADE TRADE MARKED MEETING THE REQUIREMENTS OF THE LATEST EDITION OF U.S. PRODUCT STANDARD PS-1.
- CONSTRUCTION GLUE: PL400 HEAVY DUTY CONSTRUCTION ADHESIVE BY CONTECH OR EQUAL MEETING APA SPECIFICATION AFG-01. APPLY IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS.
- UNLESS OTHERWISE NOTED ON DRAWINGS, NAIL PLYWOOD TO FRAMING AT 6" O/C ON EDGES OF SHEET AND 12" O/C ON EACH INTERIOR SUPPORT.
- FOR PLYWOOD 1/2" IN THICKNESS AND LESS USE H CLIPS AT MIDPOINT FOR SPANS GREATER THAN 16" O/C. FOR PLYWOOD 5/8" AND THICKER USE T & G EDGES OR H CLIPS AT MIDPOINT FOR SPANS GREATER THAN 16" O/C.
- UNLESS NOTED OTHERWISE, STRUCTURAL LUMBER TO BE NO.1/NO.2 SPRUCE-PINE-FIR (SPF) OR NO.2 SOUTHERN YELLOW PINE (SYP) OR BETTER AS SPECIFIED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION (NFPA) (19% MOISTURE CONTENT IN USE). PROVIDE PRESSURE TREATED LUMBER WHERE EXPOSED OR IN CONTACT WITH SOIL, CONCRETE, OR MASONRY.
- TREAT WOOD PLATES IN CONTACT WITH MASONRY IN COMPLIANCE WITH AMERICAN WOOD PRESERVERS BUREAU STANDARDS LP-2 AND LP-22.
- LAMINATED VENEER LUMBER (LVL) SHALL HAVE AN ALLOWABLE BENDING STRESS OF 2,950 PSI, AN ALLOWABLE MODULUS OF ELASTICITY OF 2,000 KSI, AN ALLOWABLE SHEAR STRESS OF 290 PSI, AND ALLOWABLE COMPRESSIVE STRESS PARALLEL TO THE GRAIN OF 2,900 PSI.
- WHERE CONNECTIONS OR OTHER DETAILS ARE NOT INDICATED, FOLLOW THE RECOMMENDATIONS IN THE MANUAL OF WOOD FRAME CONSTRUCTION, BY THE NFPA.
- SHOP DRAWINGS SHOWING ALL SECTIONS, DETAILS, AND MATERIAL SPECIFICATIONS NECESSARY FOR THE PROPER INSTALLATION AND CONNECTION OF ALL MANUFACTURER'S WOOD PRODUCTS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO FABRICATION AND INSTALLATION.

STEEL LINTELS

- PROVIDE LINTELS OVER ALL PENETRATIONS IN MASONRY WALLS AT DOORS, WINDOWS, MECHANICAL AND ELECTRICAL SERVICES AND EQUIPMENT, ETC., AND AT LOCATIONS NOT SPECIFICALLY SHOWN ON THE DRAWINGS, IN ACCORDANCE WITH THE LINTEL SCHEDULE.
- CONTRACTOR SHALL SHORE ALL LINTELS AS REQUIRED TO PREVENT ROTATION DURING CONSTRUCTION.
- CONTRACTOR SHALL COORDINATE SIZE AND TYPE OF LINTEL WITH ARCHITECTURAL, PLUMBING, MECHANICAL AND ELECTRICAL DRAWINGS.
- ALL LINTELS TO HAVE SOLID BEARING, 8" MINIMUM, EACH END.
- ALL LINTELS LOCATED IN EXTERIOR WALLS SHALL BE GALVANIZED.

WOOD LINTELS

- FOR ALL WALL OPENINGS NOT SPECIFICALLY DETAILED, PROVIDE A WOOD LINTEL PER THE FOLLOWING SCHEDULE.
- PROVIDE A DOUBLE STUD EACH END OF ALL WL-1 AND WL-2 LINTELS. PROVIDE A TRIPLE STUD EACH END OF ALL WL-3 LINTELS.

STEEL LINTEL SCHEDULE

MARK	SIZE	COMMENTS
L1	L4 X 3-1/2 X 5/16 LLV FOR EACH 4" OF WALL THICKNESS. UP TO 6'-0" OPENINGS	FOR CAVITY WALLS, USE 1-L5 X 5 X 5/16 LLV FOR VENEER.
L2	L6 X 3-1/2 X 5/16 LLV FOR EACH 4" OF WALL THICKNESS. 6'-1" TO 8'-0" OPENINGS	FOR CAVITY WALLS, USE 1-L5 X 5 X 5/16 LLV FOR VENEER.

WOOD LINTEL SCHEDULE

MARK	SIZE	# JACK	# KING	COMMENTS
WL1	3-2X6	1-2X	1-2X	FOR OPENINGS UP TO 4'-0" UNO ON PLAN
WL2	3-2X8	2-2X	2-2X	SEE PLAN
WL3	3-1.75"x11.25" LVL	2-2X	2-2X	SEE PLAN

NOTE: LINTEL STUDS TO MATCH WALL STUD SIZE, SEE PLAN.

P.T. SILL PLATE TO CONCRETE ATTACHMENT SCHEDULE

ATTACHMENT METHOD	LOCATION		
	AT EXTERIOR OF BUILDING (EXCEPT BASEMENT WALLS)	AT INTERIOR LOAD BEARING WALLS	AT ALL NON LOAD BEARING WALLS
1/2" DIAM. ANCHORx11" LONG + 2 BEND. (7" MIN. EMBEDMENT)	4'-0"/c	4'-0"/c	6'-0"/c
POWDER-ACTUATED FASTENERS 0.177" DIA.x3" (1-1/2" MIN.) PENETRATION INTO CONCRETE			2'-0"/c*
1/2" DIAM. EXPANSION BOLTS (4" EMBEDMENT)	4'-0"/c	4'-0"/c	4'-0"/c

- BEGIN SPACING WITH A BOLT OR FASTENER 6" FROM THE END OF THE PLATE
- * THESE FASTENERS TO HAVE 36mm Ø WASHERS. BEGIN SPACING WITH A FASTENER 6" FROM THE END OF THE PLATE.
- ALL SILL PLATES TO BE PRESSURE TREATED (P.T.). CONTRACTOR TO ENSURE THAT P.T. WOOD HAS BEEN PROPERLY AIR-SEASONED OR KILN-DRIED.

STRUCTURAL ABBREVIATIONS

AFF	ABOVE FINISHED FLOOR ARCHITECTURAL DRAWINGS
ARCH	
BFF	BELOW FINISHED FLOOR
BOD	BOTTOM OF DECK
B/	BOTTOM OF
BRG	BEARING
CJ	CONTROL JOINT
COORD	COORDINATE
DTL	DETAIL
DWG	DRAWING
EJ	EXPANSION JOINT
ELEV	ELEVATION
EXIST	EXISTING
EM	CALCULATED END MOMENT FOR WHICH MEMBER AND CONNECTION SHALL BE DESIGNED
EOS	EDGE OF SLAB
FTG	FOOTING
IJ	ISOLATION JOINT
MANUF	MANUFACTURER
MB	MASONRY BEARING
MC	MOMENT RESISTING CONNECTION. MOMENT VALUE (IF STATED) IS IN UNITS OF FT-KIPS. SEE TYPICAL DETAIL
MCJ	MCJ
OC, o/c	ON CENTER
PL	PLATE
R	CALCULATED END REACTION FOR WHICH THE CONNECTION SHALL BE DESIGNED
RTU	ROOFTOP UNIT
SEE	FOR ADDITIONAL CONSTRUCTION REQUIREMENTS, REFER TO
SJ	STRUT JOIST
STD	STANDARD
SC	STIFFENED SEAT CONNECTION
T/	TOP OF
T/BRG	TOP OF STEEL BEARING PLATE
T/S, T.O.S.	TOP OF STEEL
TDS	TURN-DOWN SLAB
T/F	TOP OF FOOTING
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VIF	VERIFY IN FIELD



PROFESSIONAL CERTIFICATION LICENSE NUMBER: THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND, LICENSE NO. 21185, EXPIRATION DATE 9/31/21.
SEAL AND DATE

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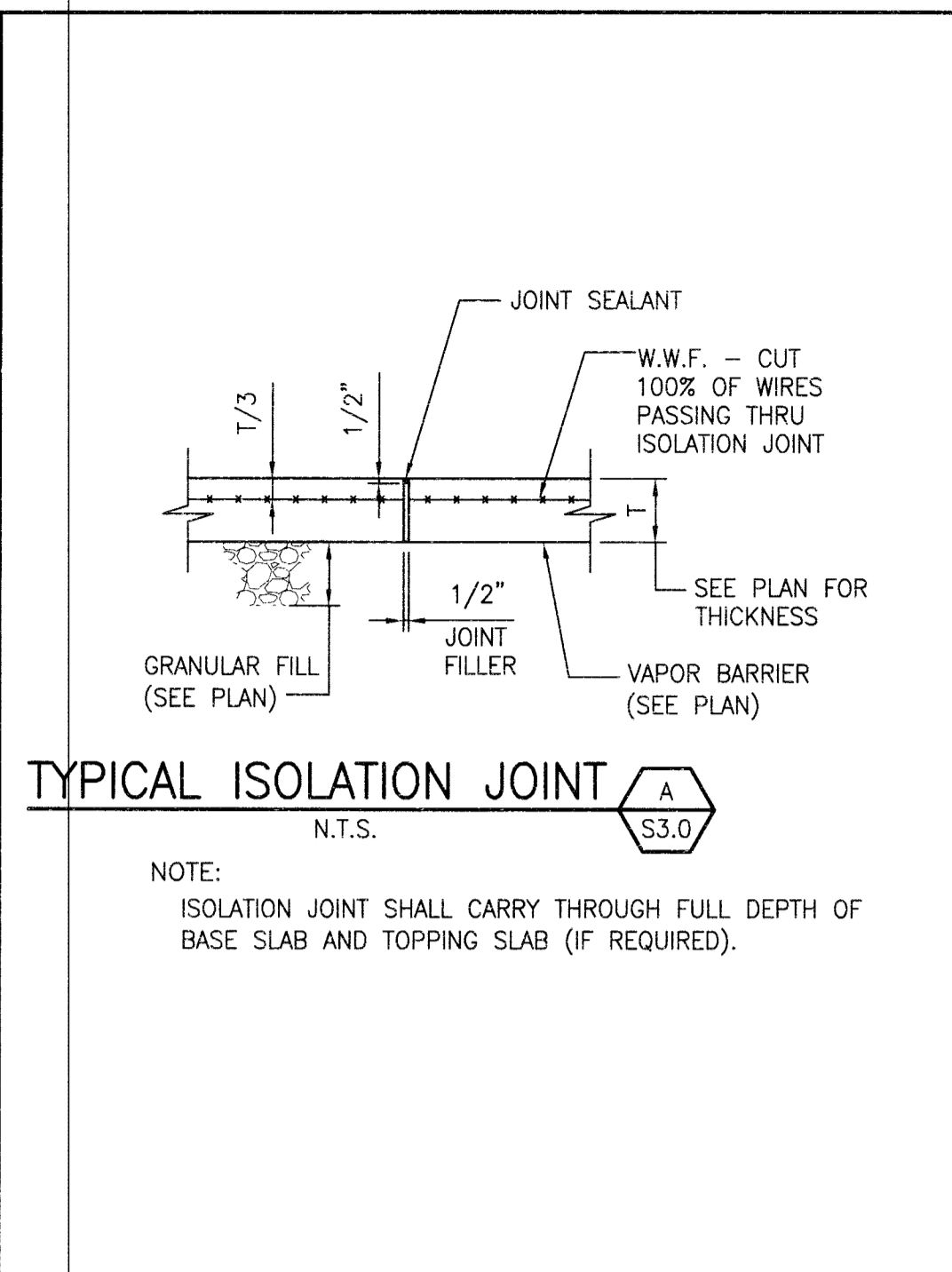
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REVISIONS

NO.	DATE	DESCRIPTION

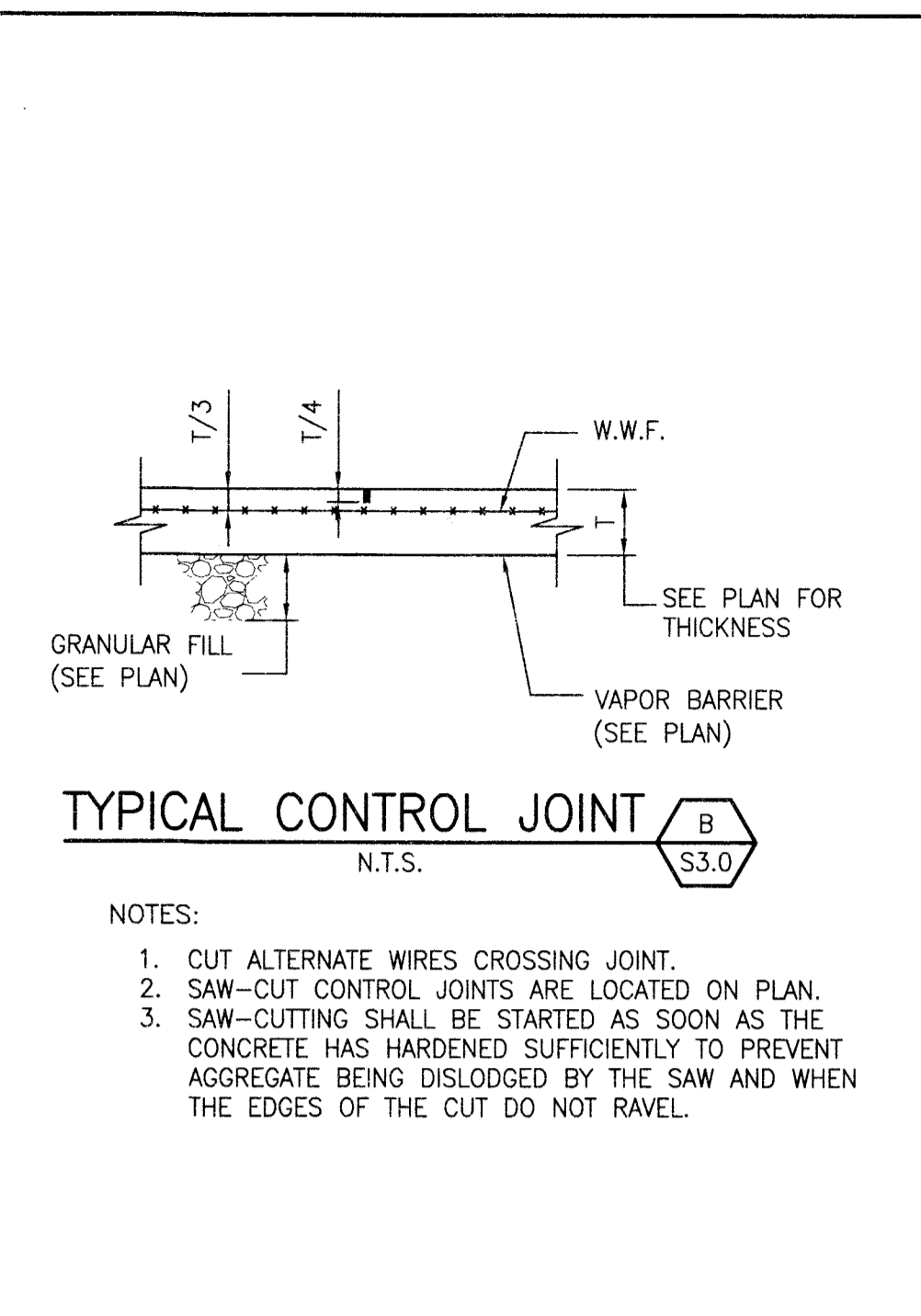
TYPICAL DETAILS

SCALE AS NOTED SHEET NUMBER
 DATE MAY 26, 2010 **S3.0**
 PROJECT NUMBER 0.907



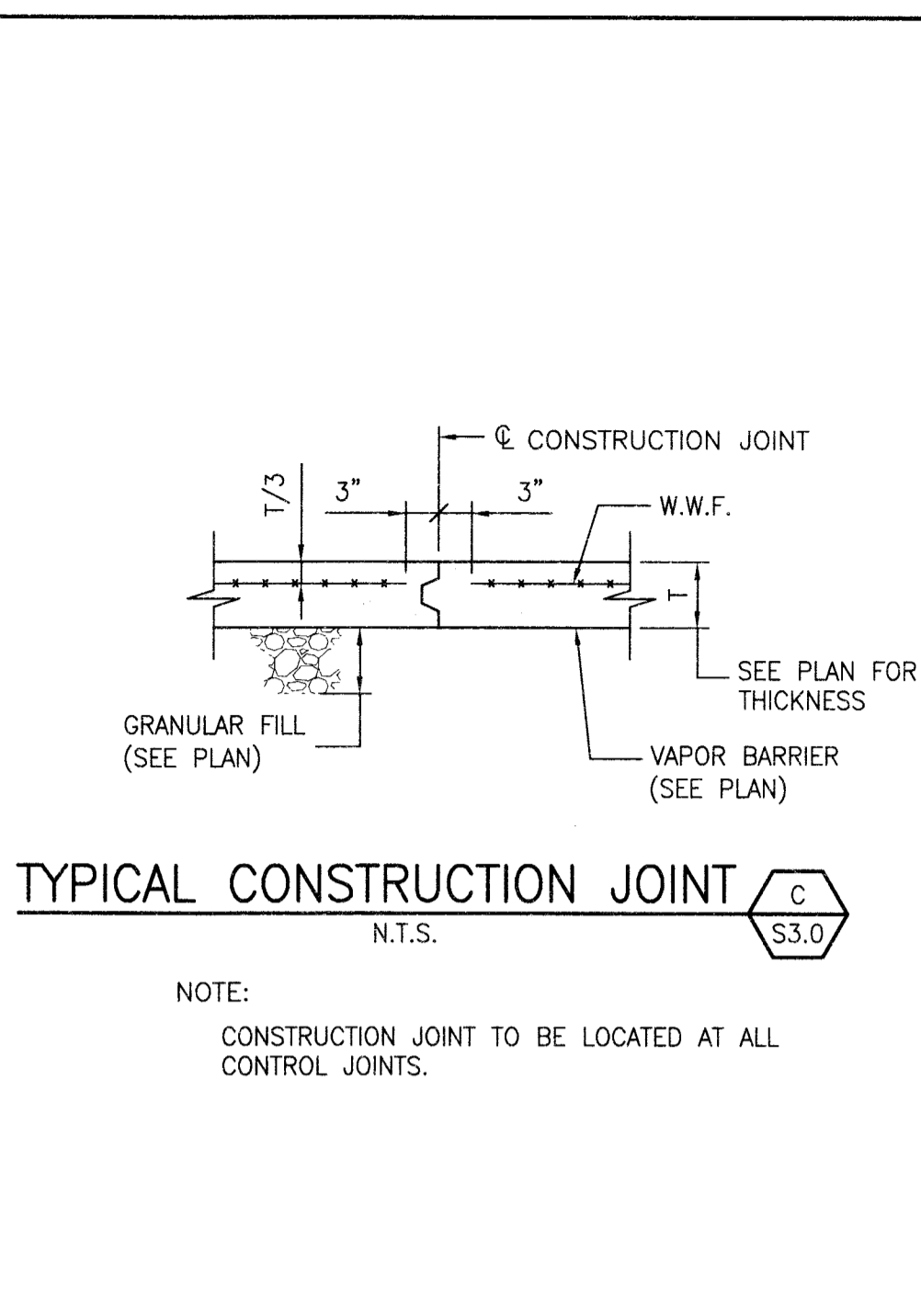
TYPICAL ISOLATION JOINT A S3.0
 N.T.S.

NOTE:
 ISOLATION JOINT SHALL CARRY THROUGH FULL DEPTH OF BASE SLAB AND TOPPING SLAB (IF REQUIRED).



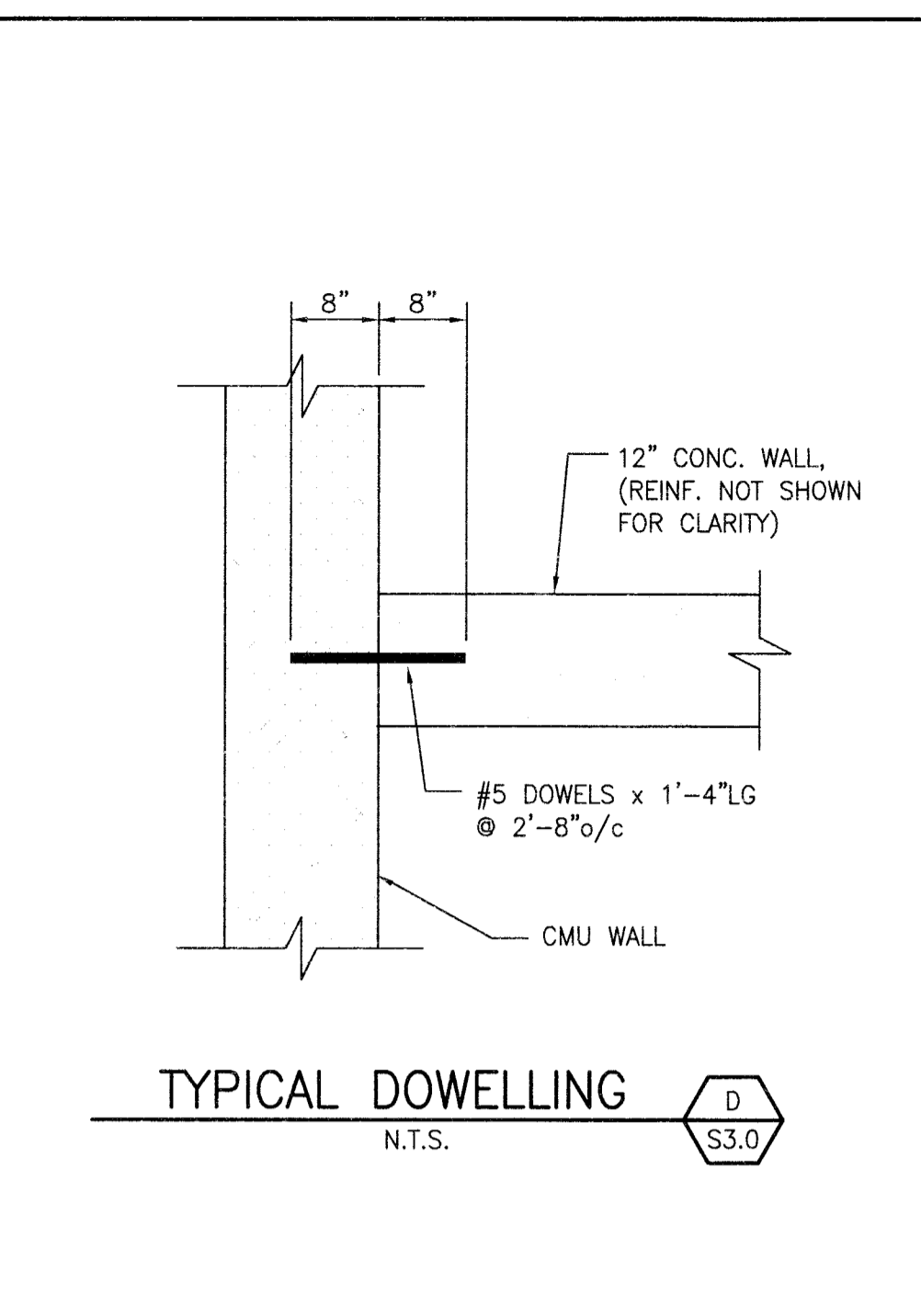
TYPICAL CONTROL JOINT B S3.0
 N.T.S.

NOTES:
 1. CUT ALTERNATE WIRES CROSSING JOINT.
 2. SAW-CUT CONTROL JOINTS ARE LOCATED ON PLAN.
 3. SAW-CUTTING SHALL BE STARTED AS SOON AS THE CONCRETE HAS HARDENED SUFFICIENTLY TO PREVENT AGGREGATE BEING DISLODGED BY THE SAW AND WHEN THE EDGES OF THE CUT DO NOT RAVEL.

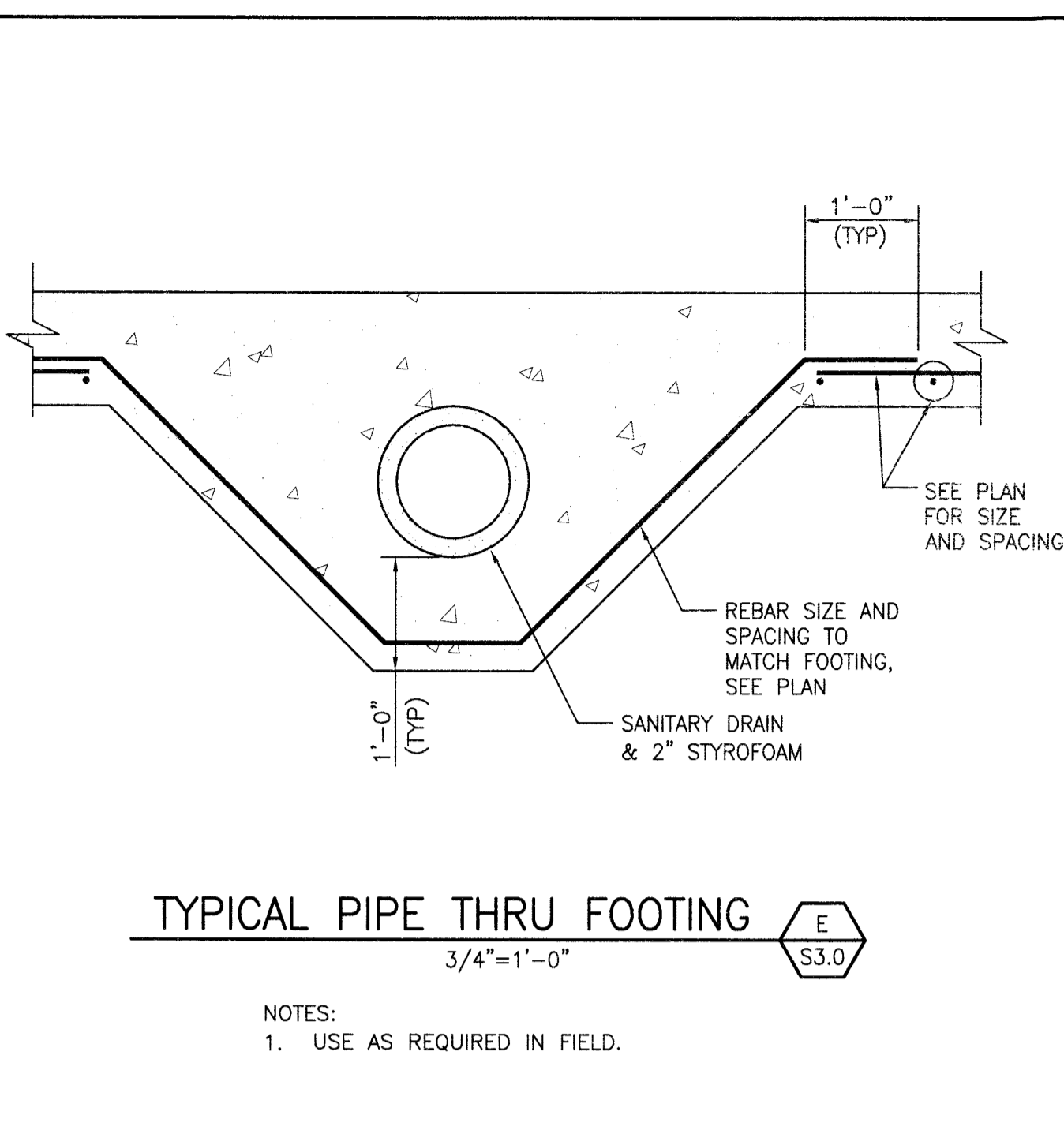


TYPICAL CONSTRUCTION JOINT C S3.0
 N.T.S.

NOTE:
 CONSTRUCTION JOINT TO BE LOCATED AT ALL CONTROL JOINTS.

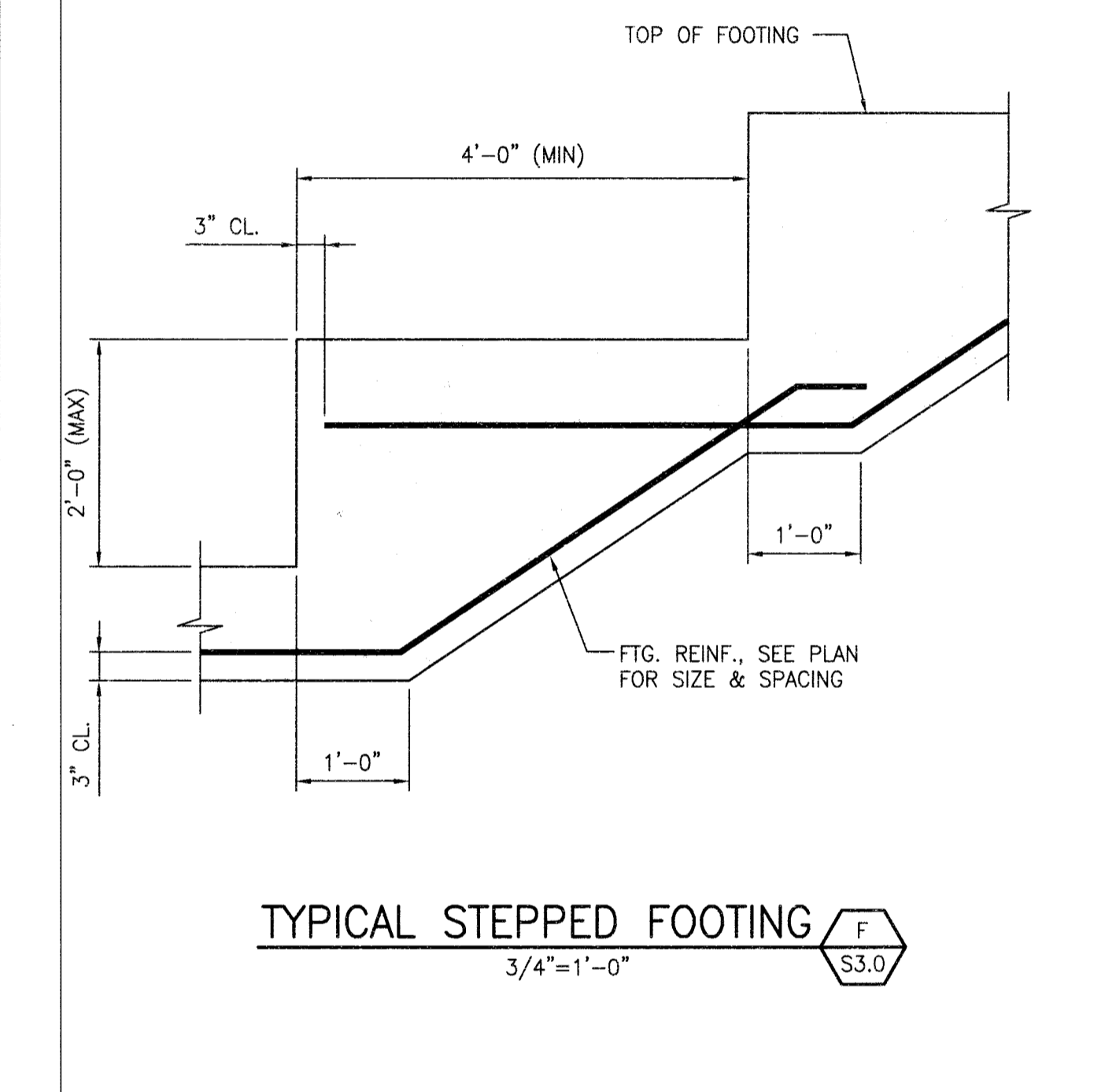


TYPICAL DOWELLING D S3.0
 N.T.S.

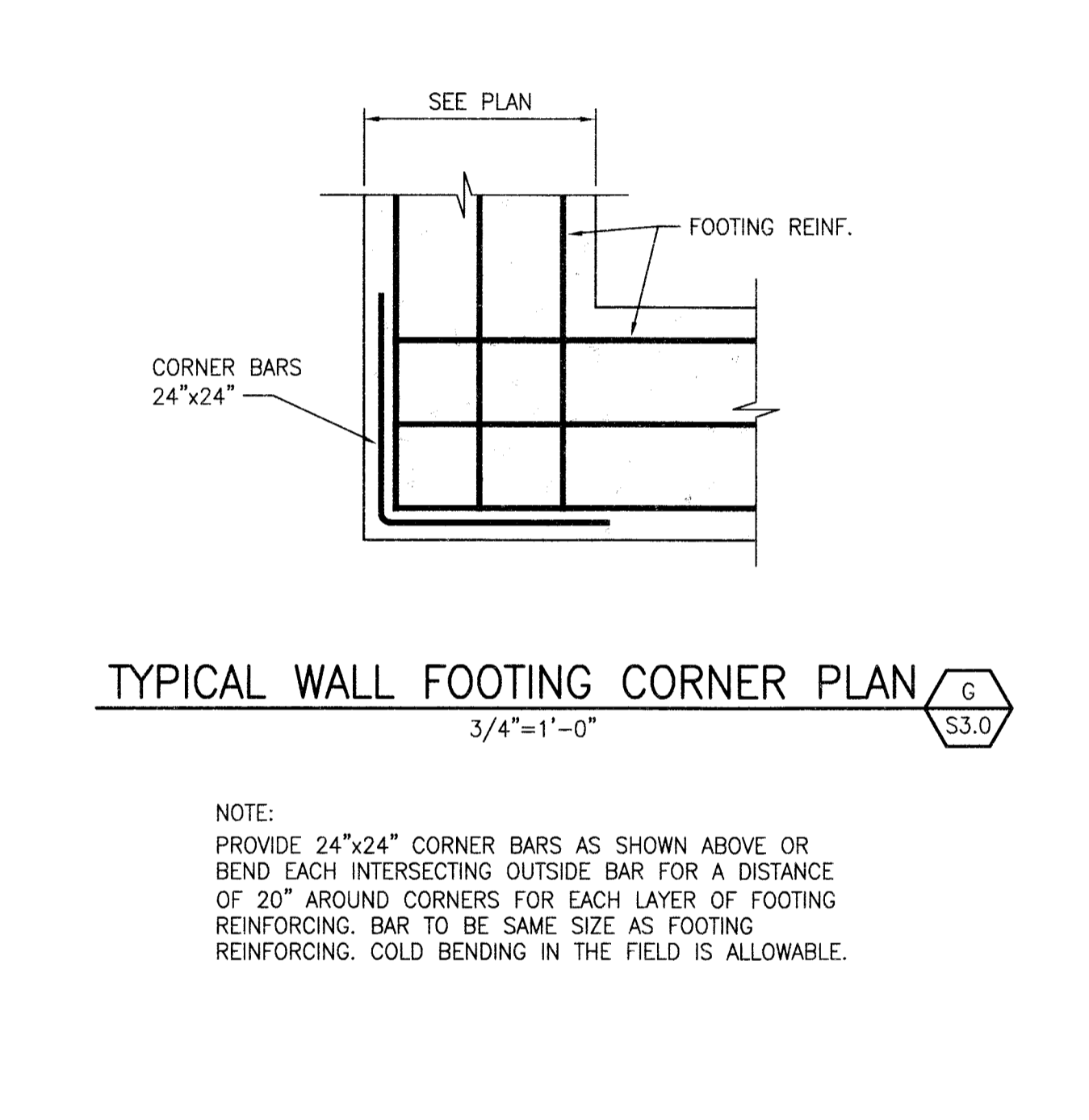


TYPICAL PIPE THRU FOOTING E S3.0
 3/4\"=1'-0\"

NOTES:
 1. USE AS REQUIRED IN FIELD.

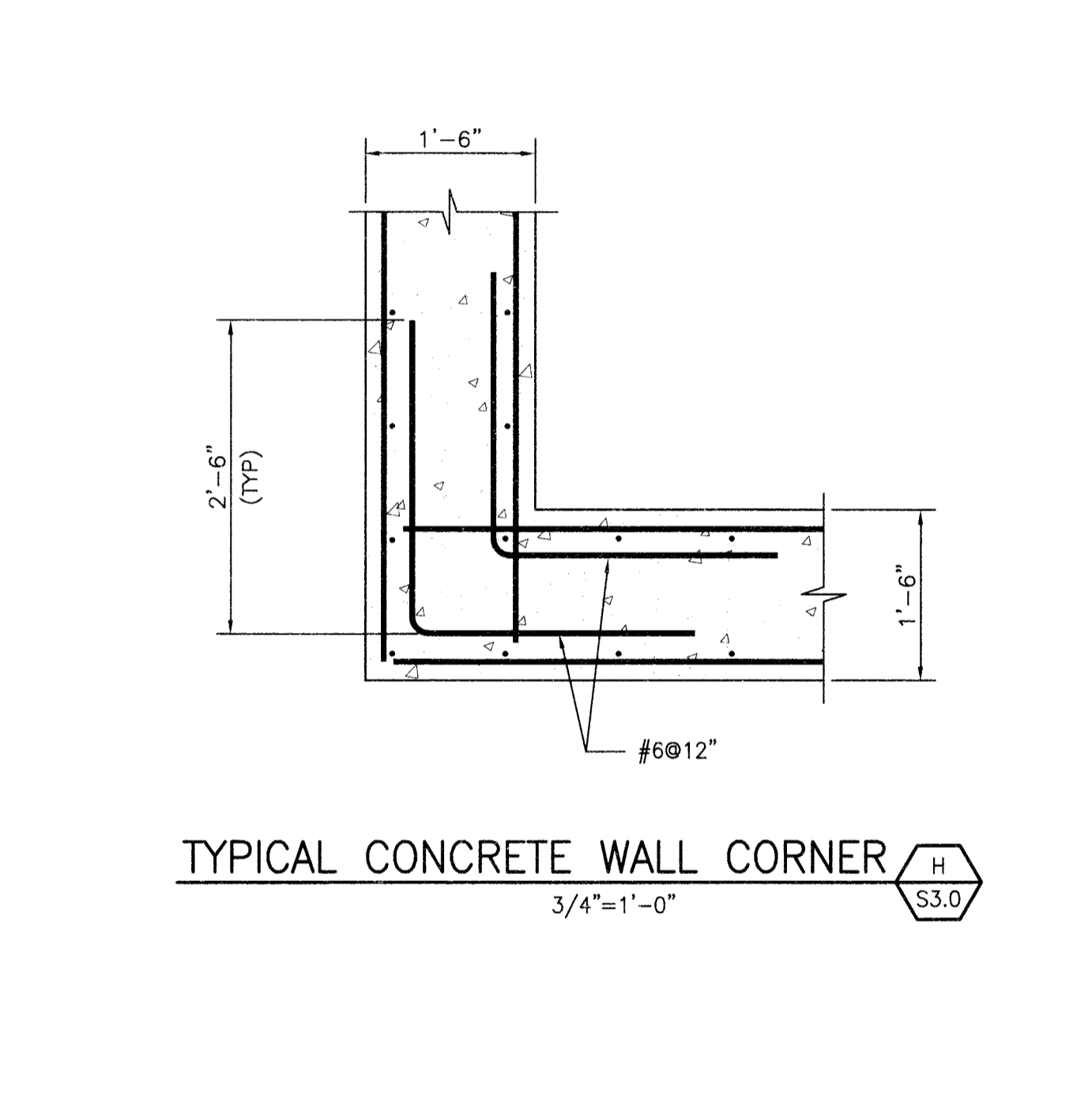


TYPICAL STEPPED FOOTING F S3.0
 3/4\"=1'-0\"

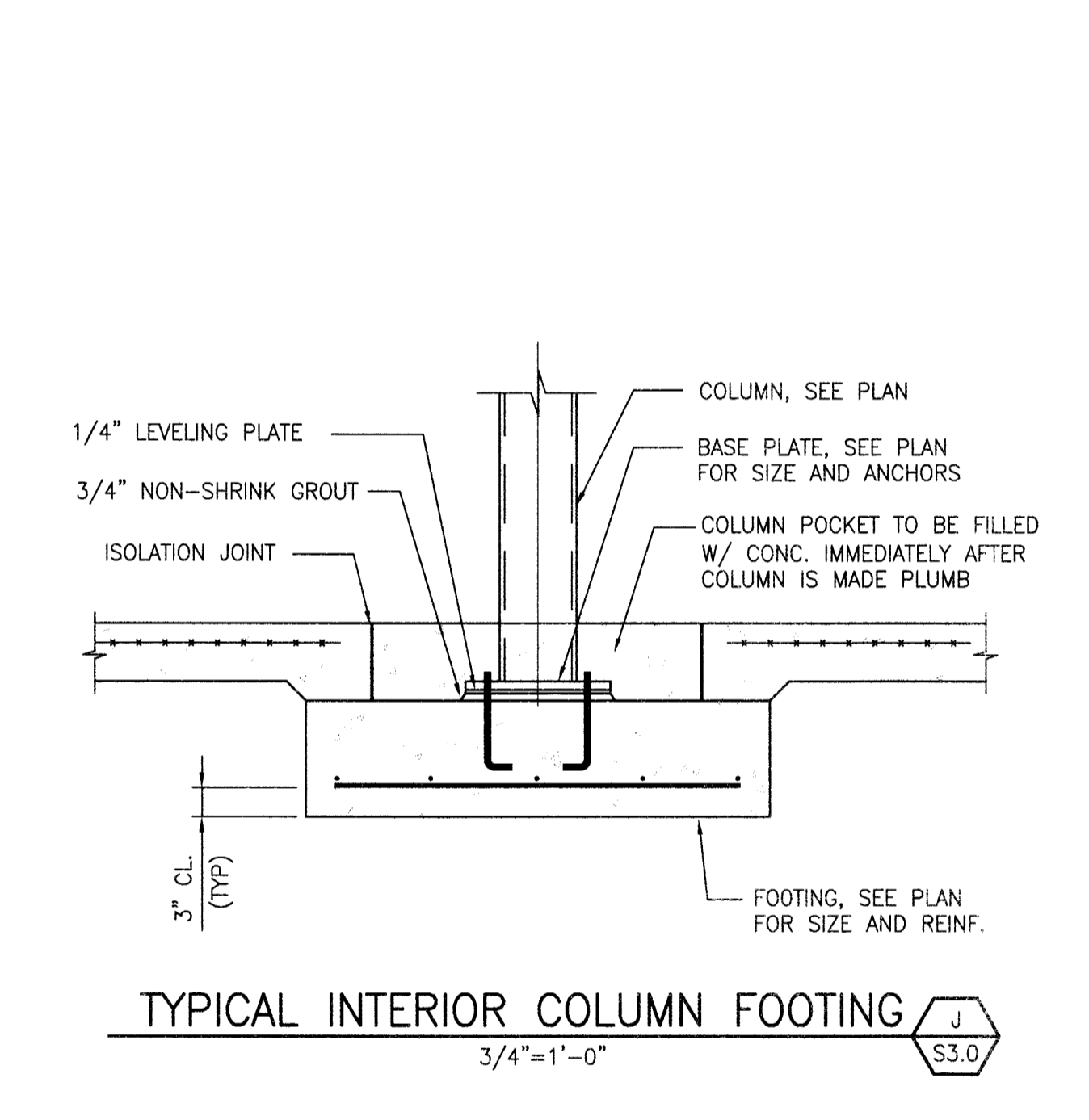


TYPICAL WALL FOOTING CORNER PLAN G S3.0
 3/4\"=1'-0\"

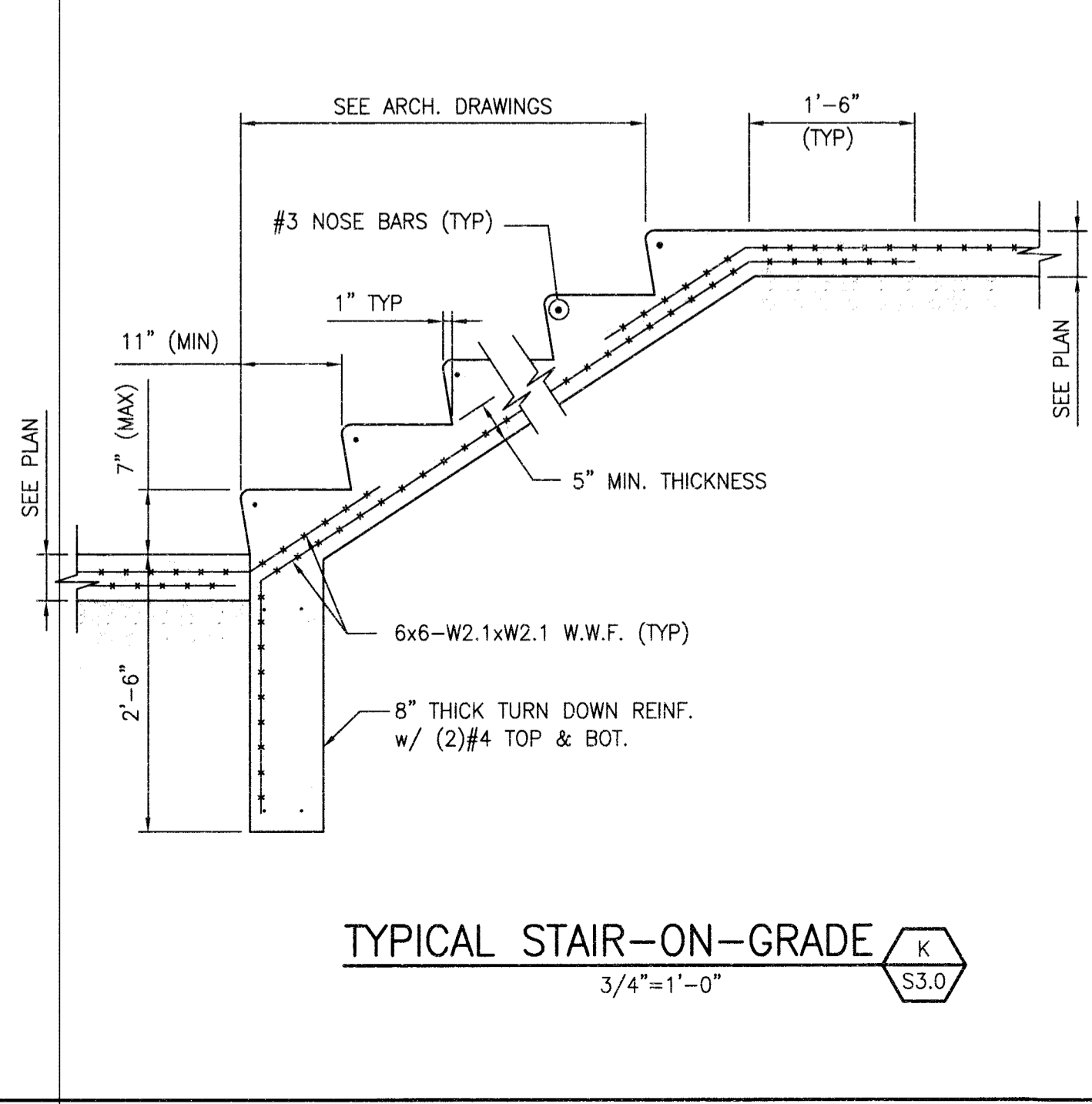
NOTE:
 PROVIDE 24\"x24\" CORNER BARS AS SHOWN ABOVE OR BEND EACH INTERSECTING OUTSIDE BAR FOR A DISTANCE OF 20\" AROUND CORNERS FOR EACH LAYER OF FOOTING REINFORCING. BAR TO BE SAME SIZE AS FOOTING REINFORCING. COLD BENDING IN THE FIELD IS ALLOWABLE.



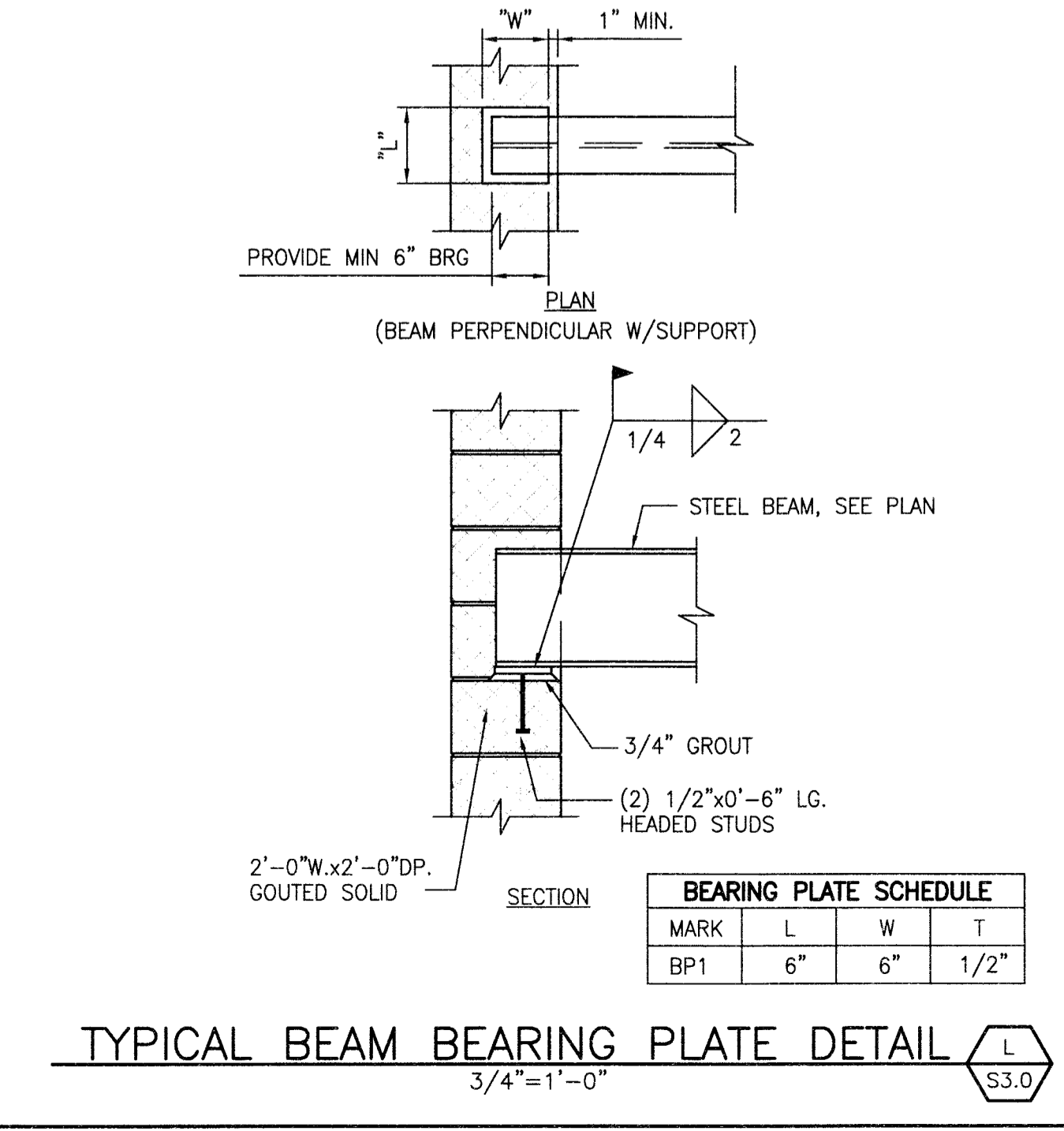
TYPICAL CONCRETE WALL CORNER H S3.0
 3/4\"=1'-0\"



TYPICAL INTERIOR COLUMN FOOTING J S3.0
 3/4\"=1'-0\"

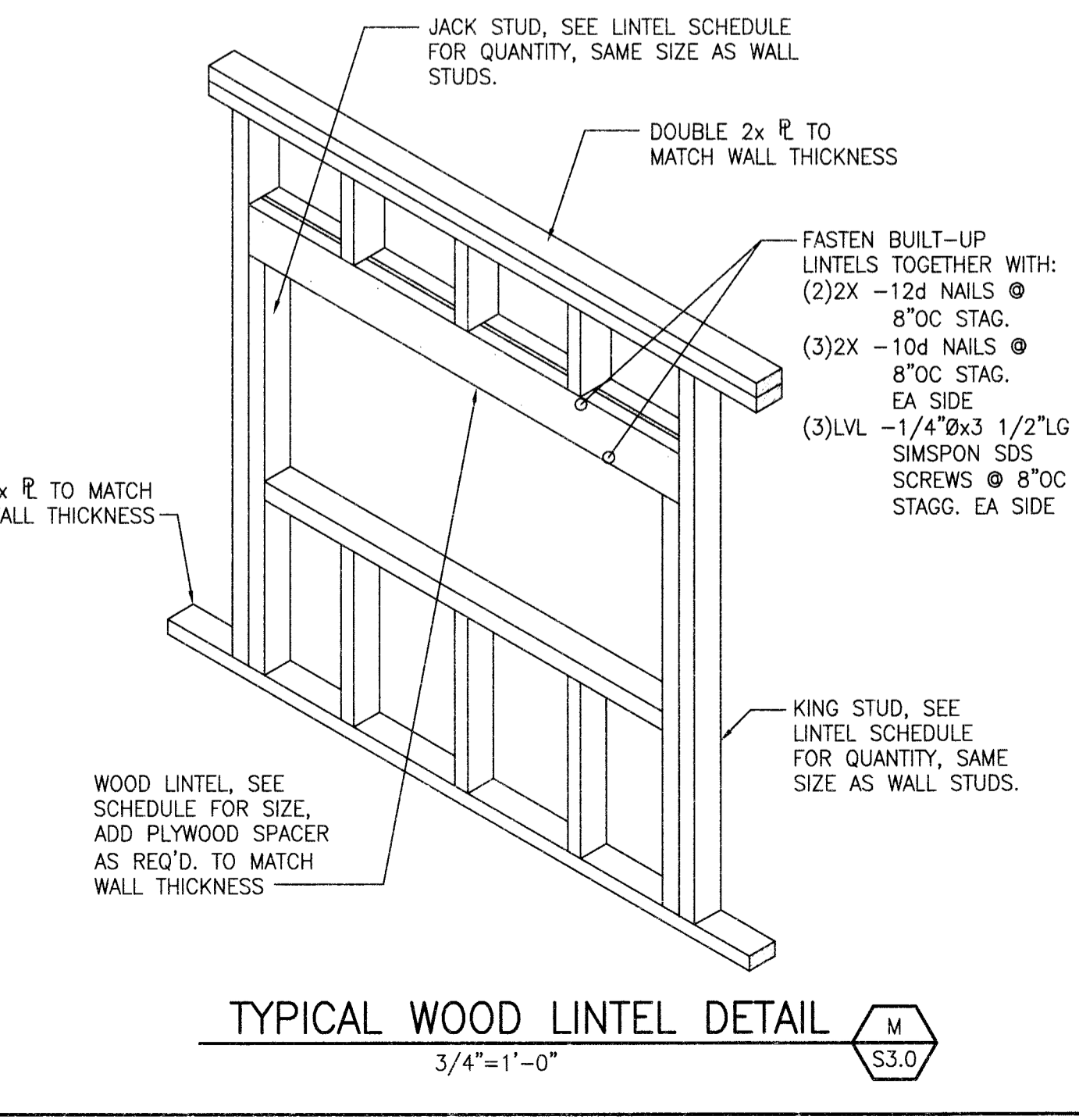


TYPICAL STAIR-ON-GRADE K S3.0
 3/4\"=1'-0\"

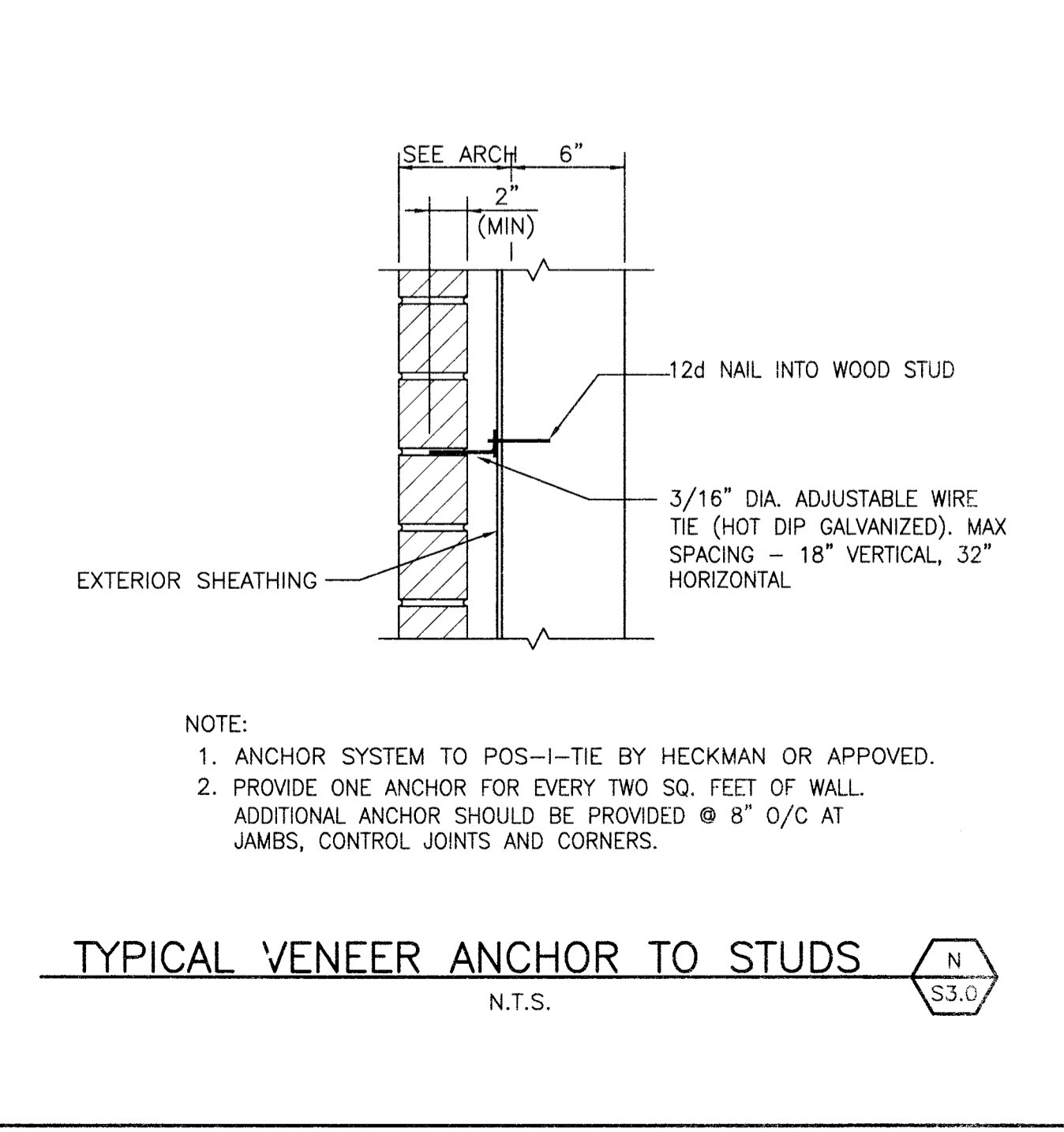


TYPICAL BEAM BEARING PLATE DETAIL L S3.0
 3/4\"=1'-0\"

BEARING PLATE SCHEDULE			
MARK	L	W	T
BP1	6"	6"	1/2"



TYPICAL WOOD LINTEL DETAIL M S3.0
 3/4\"=1'-0\"



TYPICAL VENEER ANCHOR TO STUDS N S3.0
 N.T.S.

NOTE:
 1. ANCHOR SYSTEM TO POS-I-TIE BY HECKMAN OR APPROVED.
 2. PROVIDE ONE ANCHOR FOR EVERY TWO SQ. FEET OF WALL. ADDITIONAL ANCHOR SHOULD BE PROVIDED @ 8\"/>



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PROFESSIONAL CERTIFICATION UNDER THE PROFESSIONAL ENGINEERS AND SURVEYORS ACT OF 1971. THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND, LICENSE NO. 2176, EXPIRATION DATE 3/31/12.
 SEAL AND DATE

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REVISIONS

NO.	DESCRIPTION

SECTIONS

SCALE AS NOTED SHEET NUMBER
 DATE MAY 26, 2010 **S4.1**
 PROJECT NUMBER 0.907

